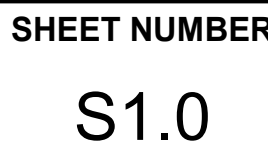


Sheet List	
Sheet Number	Sheet Name
S1.0	COVER SHEET
S1.1	GENERAL NOTES
S1.2	SPECIAL INSPECTIONS
S1.3	TYPICAL DETAILS
S2.0	FOUNDATION PLAN & CATWALK FRAMING PLAN - LEVEL 1
S2.1	CATWALK FRAMING PLANS - LEVELS 2 - 4
S3.0	ELEVATION & SECTION
S3.1	SECTIONS



INITIAL PRICING SET - NOT FOR CONSTRUCTION

General Notes

GN.1	<p>THE STRUCTURAL DRAWINGS AND SPECIFICATIONS ARE A PORTION OF THE CONSTRUCTION DOCUMENTS. THE CONTRACTOR AND SUBCONTRACTORS SHALL REFERENCE AND COORDINATE WITH ALL OTHER DISCIPLINES' DRAWINGS. ANY DISCREPANCIES OR OMISSIONS SHALL BE REPORTED TO THE STRUCTURAL ENGINEER AND ARCHITECT.</p>								
GN.2	<p>DESIGN CRITERIA:</p> <p>A. CODES AND SPECIFICATIONS:</p> <ol style="list-style-type: none"> GENERAL BUILDING CODE: FLORIDA BUILDING CODE, ____ EDITION DESIGN LOAD CRITERIA: MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES, AMERICAN SOCIETY OF CIVIL ENGINEERS, ASCE 7 CONCRETE: BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE, AMERICAN CONCRETE INSTITUTE, ACI 318. ARCHITECTURAL PRECAST CONCRETE: MANUAL FOR QUALITY CONTROL FOR PLANTS AND PRODUCTION OF ARCHITECTURAL PRECAST CONCRETE PRODUCTS, PRECAST/PRESTRESSED CONCRETE INSTITUTE, PCI, MNL 117. STRUCTURAL STEEL: SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS, AMERICAN INSTITUTE OF STEEL CONSTRUCTION, AISC 360. <p>B. DESIGN LOADS (PSF):</p> <ol style="list-style-type: none"> DEAD LOADS: ANY CHANGES IN CONSTRUCTION MATERIALS FROM THOSE SHOWN ON THE ARCHITECTURAL OR STRUCTURAL DRAWINGS SHALL BE REPORTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER FOR VERIFICATION OF LOAD-CARRYING CAPACITY OF THE STRUCTURE. WIND LOADS: BASIC WIND SPEED (3 – SECOND GUST)-----115 MPH WIND IMPORTANCE FACTOR (Iw)-----1.0 BUILDING CATEGORY-----II WIND EXPOSURE CATEGORY-----B INTERNAL PRESSURE COEFFICIENT-----N/A (SIGN) WALL COMPONENT AND CLADDING WIND PRESSURE SHALL BE 50 PSF (ULTIMATE) FOR ALL SURFACES. SEISMIC LOADS: SEISMIC IMPORTANCE FACTOR (Ie)-----1.0 MAPPED SPECTRAL RESPONSE ACCELERATIONS: Ss-----0.261 S1-----0.104 SITE CLASS-----B SITE COEFFICIENTS: Fa-----1.591 Fv-----2.383 DESIGN SPECTRAL RESPONSE ACCELERATION PARAMETERS: Sds-----0.174 Sd-----0.069 SEISMIC DESIGN CATEGORY-----B BASIC SEISMIC-FORCE-RESISTING SYSTEM: DESIGN BASE SHEAR-----7 KIPS SEISMIC RESPONSE COEFFICIENT (Cs)-----0.058 RESPONSE MODIFICATION FACTOR(R)-----3 OVER-STRENGTH FACTOR (po)-----3 DEFLECTION AMPLIFICATION FACTOR (Cd)-----3 ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE METHOD 								
GN.3	<p>CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND SITE CONDITIONS PRIOR TO FABRICATION/CONSTRUCTION. NOTIFY STRUCTURAL ENGINEER AND ARCHITECT OF ANY DISCREPANCIES PRIOR TO FABRICATION/CONSTRUCTION.</p>								
GN.4	<p>SPECIAL INSPECTIONS/STRUCTURAL ENGINEER'S SITE VISITS:</p> <p>A. SPECIAL INSPECTIONS ARE REQUIRED FOR THIS PROJECT IN ACCORDANCE WITH INTERNATIONAL BUILDING CODE. REFER TO DRAWINGS.</p> <p>B. SITE VISITS BY STRUCTURAL ENGINEER:</p> <ol style="list-style-type: none"> STRUCTURAL ENGINEER'S SITE VISITS ARE FOR VISUAL OBSERVATION OF THE IN-PLACE STRUCTURE FOR GENERAL CONFORMANCE TO THE APPROVED CONSTRUCTION DOCUMENTS AT THE TIME OF THE OBSERVATION. CONTRACTOR SHALL NOTIFY STRUCTURAL ENGINEER AND ARCHITECT, PER THE SCHEDULE STATED BELOW, WHEN SUCH ITEMS HAVE PROGRESSED TO THE POINT WHERE THEY WILL BE IN PLACE AND READY FOR REVIEW. FAILURE TO NOTIFY MAY REQUIRE REMOVAL OF COMPLETED CONSTRUCTION. <table> <tr> <th data-bbox="329 1577 507 1604">NOTIFY PRIOR TO THE FOLLOWING SCHEDULED TASKS</th><th data-bbox="641 1577 733 1604">REQUIRED DAYS NOTIFICATION</th></tr> <tr> <td data-bbox="329 1633 623 1648">FIRST FOUNDATION POUR-----</td><td data-bbox="641 1633 733 1648">2 DAYS</td></tr> <tr> <td data-bbox="329 1648 623 1663">FIRST INSTALLATION OF HORIZONTAL GIRTS-----</td><td data-bbox="641 1648 733 1663">2 DAYS</td></tr> <tr> <td data-bbox="329 1663 623 1677">SUBSTANTIAL COMPLETION OF STEEL FRAMING-----</td><td data-bbox="641 1663 733 1677">2 DAYS</td></tr> </table> <p>C. SITE VISITS BY THE STRUCTURAL ENGINEER'S OFFICE DO NOT REPLACE INSPECTIONS AND TESTING BY THE TESTING AGENCY OR SPECIAL INSPECTOR.</p>	NOTIFY PRIOR TO THE FOLLOWING SCHEDULED TASKS	REQUIRED DAYS NOTIFICATION	FIRST FOUNDATION POUR-----	2 DAYS	FIRST INSTALLATION OF HORIZONTAL GIRTS-----	2 DAYS	SUBSTANTIAL COMPLETION OF STEEL FRAMING-----	2 DAYS
NOTIFY PRIOR TO THE FOLLOWING SCHEDULED TASKS	REQUIRED DAYS NOTIFICATION								
FIRST FOUNDATION POUR-----	2 DAYS								
FIRST INSTALLATION OF HORIZONTAL GIRTS-----	2 DAYS								
SUBSTANTIAL COMPLETION OF STEEL FRAMING-----	2 DAYS								
GN.5	<p>SUBMITTALS:</p> <p>A. REVIEW OF SHOP DRAWINGS AND OTHER SUBMITTALS BY THE STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY TO REVIEW AND CHECK SHOP DRAWINGS BEFORE SUBMITTING TO THE STRUCTURAL ENGINEER. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS. ALL SHOP DRAWINGS MUST BE REVIEWED AND "APPROVED" BY THE CONTRACTOR PRIOR TO SUBMITTAL.</p> <p>B. ELECTRONIC SHOP DRAWING SUBMITTALS: SUBMIT ALL ELECTRONIC SHOP DRAWINGS IN .PDF FORMAT. REVIEWED SHOP DRAWINGS WILL BE RETURNED IN .PDF FORMAT. ALL PRINTS REQUIRED BY THE CONTRACTOR ARE THE RESPONSIBILITY OF THE CONTRACTOR AND SHALL BE MADE AFTER APPROVED SHOP DRAWINGS ARE RETURNED.</p>								

C. RESUBMITTED SHOP DRAWINGS: RESUBMITTED SHOP DRAWINGS SHALL HAVE ALL CHANGES SINCE THE PREVIOUS SUBMISSION IDENTIFIED BY CLOUDING OR OTHER CLEAR COMMUNICATION. RE-REVIEWED SHOP DRAWINGS WILL ONLY BE REVIEWED FOR IDENTIFIED CHANGES.

D. SHOP DRAWINGS: THE CONTRACTOR SHALL SUBMIT FOR STRUCTURAL ENGINEER REVIEW SHOP DRAWINGS FOR THE FOLLOWING ITEMS. ITEMS MARKED (*) SHALL HAVE SHOP DRAWINGS SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE IN WHICH THE PROJECT IS LOCATED. ITEMS MARKED (#) SHALL BE SUBMITTED FOR STRUCTURAL ENGINEER'S RECORD ONLY.

1. CONCRETE MIX DESIGNS
2. CONCRETE REINFORCING
3. STRUCTURAL STEEL
4. STRUCTURAL STEEL PAINT TECHNICAL DATA
5. VIDEOBOARD (*)
6. FALL PROTECTION SYSTEMS

E. DESIGN CALCULATIONS: THE CONTRACTOR SHALL SUBMIT FOR STRUCTURAL ENGINEER'S RECORD, DESIGN CALCULATIONS SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE IN WHICH THE PROJECT IS LOCATED FOR THE FOLLOWING ITEMS.

1. VIDEOBOARD ATTACHMENT TO STRUCTURE
2. FALL PROTECTION SYSTEMS

GN.6 ALL DETAILS SHOWN ARE TYPICAL. SIMILAR DETAILS APPLY TO SIMILAR CONDITIONS, UNLESS NOTED.

GN.7 THE CONTRACTOR IS RESPONSIBLE FOR MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION.

GN.8 CONSTRUCTION MATERIALS SHALL BE SPREAD OUT IF PLACED ON FRAMED FLOORS/ROOFS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT LOADS DO NOT EXCEED THE DESIGN LIVE LOAD.

FD. FOUNDATION

FD.1 GEOTECHNICAL REPORT: FOUNDATION DESIGN IS BASED ON THE GEOTECHNICAL REPORT BY _____ TITLED "PROPOSED SCOREBOARD AT _____" PROJECT NO. _____ DATED _____. THE CONTRACTOR SHALL OBTAIN A COPY OF THE GEOTECHNICAL REPORT FROM THE OWNER AND FOLLOW ALL REQUIREMENTS AND RECOMMENDATIONS.

FD.2 DESIGN BEARING PRESSURES (PSF):

DRILLED PIERS ON COMPETENT ROCK-----	_____ KSF
LATERAL BEARING PRESSURE-----	_____ KSF
SKIN FRICTION-----	_____ KSF

FD.3 ALL FOUNDATION BEARING SURFACES SHALL BE INSPECTED AND APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACING CONCRETE TO ENSURE COMPLIANCE WITH PRESSURES NOTED. THE FINAL BEARING ELEVATIONS MAY VARY AS REQUIRED TO PROVIDE PROPER BEARING CAPACITY IN AN APPROVED BEARING STRUTUM AS DETERMINED BY THE GEOTECHNICAL ENGINEER.

FD.4 FOOTINGS SHALL BE PLACED THE SAME DAY AS INSPECTION BY THE GEOTECHNICAL ENGINEER UNLESS EXTENDED TIME IS APPROVED BY THE GEOTECHNICAL ENGINEER.

FD.5 FOOTINGS SHALL BE NEATLY EXCAVATED WHERE POSSIBLE WITH SIDES AND TOP EDGES FREE OF LOOSE OR WET MATERIALS. WHERE NEAT EXCAVATION IS NOT POSSIBLE, FOOTING EXCAVATION SHALL BE FILLED WITH CONCRETE TO THE TOP OF FOOTING. THE BOTTOM EXCAVATION SHALL BE CLEAN AND DRY WITH ALL LOOSE MATERIAL REMOVED FOR AN ESSENTIALLY FLAT BEARING SURFACE. WHERE SOFT OR UNSUITABLE BEARING SURFACES ARE ENCOUNTERED, THE AREA SHALL BE UNDERCUT AS REQUIRED AND REPLACED WITH LEAN CONCRETE OR COMPACTED DENSE GRADED CRUSHED STONE AS DIRECTED BY THE GEOTECHNICAL ENGINEER.

CN. CONCRETE

CN.1 CONCRETING OPERATIONS SHALL COMPLY WITH ACI STANDARDS.

CN.2 MINIMUM CONCRETE COMPRESSIVE STRENGTH AT 28 DAYS (PSI), TYPE OF CONCRETE, MAXIMUM W/C (WATER/CEMENTITIOUS MATERIALS RATIO), TOTAL AIR CONTENT, SLUMP AND CONCRETE USE:

STRENGTH	TYPE	W/C	AIR	SLUMP	USE
5000	NORMAL WT.	0.57	---	6" TO 8"	UNLESS NOTED

CN.3 REINFORCING BARS: ASTM A615 GRADE 60.

CN.4 REINFORCING STEEL SHOWN IN SECTIONS AND DETAILS IS A SCHEMATIC INDICATION THAT REINFORCING EXISTS. SEE SCHEDULES, SECTION NOTES AND GENERAL NOTES FOR ACTUAL REINFORCING REQUIRED.

CN.5 REINFORCING BAR PLACING ACCESSORIES TO BE INSTALLED IN ACCORDANCE WITH ACI MANUAL OF STANDARD PRACTICE. WHERE CONCRETE IS EXPOSED IN FINISHED BUILDING, PROVIDE ACCESSORIES WITH RUSTPROOF LEGS.

CN.6 DETAIL REINFORCEMENT IN ACCORDANCE WITH ACI 315. REINFORCEMENT SHALL NOT BE WELDED UNLESS NOTED OR APPROVED BY THE STRUCTURAL ENGINEER.

CN.7 SPLICES SHALL BE CLASS "B" TENSION LAP SPLICE, UNLESS NOTED.

CN.8 REINFORCING MARKED "CONTINUOUS" SHALL BE SPLICED WITH CLASS "B" TENSION LAP SPLICE, UNLESS NOTED.

CN.9 CONCRETE COVERAGE OF REINFORCEMENT, UNLESS NOTED:

DRILLED PIERS-----	3" CLEAR OF TIES
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SS. STRUCTURAL STEEL

SS.1 FABRICATE AND ERECT ALL STRUCTURAL STEEL IN ACCORDANCE WITH AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".

SS.2 THE STEEL FRAME IS "NON-SELF-SUPPORTING". ADEQUATE TEMPORARY SUPPORT MUST BE PROVIDED BY THE CONTRACTOR UNTIL THE LATERAL FORCE RESISTING SYSTEM AND STABILITY OF THE COMPLETED STRUCTURE IS IN PLACE.

SS.3 LATERAL FORCE RESISTING SYSTEM AND STABILITY OF THE BUILDING IN THE COMPLETED STRUCTURE IS PROVIDED AS FOLLOWS:

STEEL CANTILEVERED COLUMNS NOT SPECIFICALLY DESIGNED FOR SEISMIC RESISTANCE.

SS.4	STRUCTURAL STEEL AND STRUCTURAL STEEL CONNECTIONS SHALL CONFORM TO THE FOLLOWING MINIMUM REQUIREMENTS UNLESS NOTED OTHERWISE:	
	W AND WT SHAPES	ASTM A992
	STIFFENER PLATES, BASE PLATES, CAP PLATES, CONNECTION PLATES, AND ANGLES	ASTM A36
	HOLLOW STRUCTURAL SECTIONS	ASTM A500, GRADE C
	WELDED CONNECTIONS	E70XX ELECTRODES, MINIMUM SIZE FILLET WELD 3/16"
	HEADED ANCHOR RODS	ASTM F1554 GRADE 105 ANCHOR AND HEAVY HEX NUT, UNLESS INDICATED.
	BOLTS	ASTM A325 OR A490
	NUTS	ASTM A563
	WASHERS	ASTM F436
SS.5	WHERE NO CAMBER IS INDICATED, BEAMS SHOULD BE ERECTED WITH NATURAL CAMBER ORIENTED UPWARD.	
SS.6	HSS MEMBERS SHALL HAVE A 1/4" CLOSURE PLATE.	
SS.7	GROUT UNDER BEARING PLATES SHALL BE NON-SHRINK, NON-METALLIC TYPE. GROUT SHALL HAVE A SPECIFIED DESIGN COMPRESSIVE STRENGTH TWO TIMES THAT OF THE SUPPORTING CONCRETE.	
SS.8	STRUCTURAL STEEL MEMBERS SHALL NOT BE CUT, SPLICED, OR MODIFIED IN THE FIELD UNLESS NOTED ON THE STRUCTURAL DRAWINGS OR APPROVED BY THE STRUCTURAL ENGINEER.	
SS.9	STRUCTURAL STEEL COATING SYSTEMS SHALL CONFORM TO THE REQUIREMENTS ESTABLISHED IN THE PAINT (PN) SECTION OF THESE GENERAL NOTES.	
SS.10	DRAIN HOLES SHALL BE PROVIDED IN ALL STEEL AS REQUIRED TO PREVENT WATER ACCUMULATION. HOLES THROUGH STRUCTURAL STEEL MEMBERS SHALL BE ROUND SMOOTH AND NOT EXCEEDING 1/2" DIAMETER. DRAIN HOLES SHALL BE LEFT CLEAN AND UNOBSTRUCTED.	
SC. STRUCTURAL STEEL CONNECTIONS		
SC.1	ERECTION AIDS ARE NOT SHOWN ON THESE DRAWINGS. CONTRACTOR IS TO PROVIDE ERECTION AIDS AS REQUIRED AND REMOVE THEM ONCE WORK IS COMPLETE.	
SC.2	ALL WELDS SHALL CONFORM TO THE AMERICAN WELDING SOCIETY (ANSI/AWS D1.1) STANDARDS AND MUST BE PERFORMED BY AN ANSI/AWS CERTIFIED WELDER.	
SC.3	ALL WELD SIZES ARE TO BE CONSIDERED AS EFFECTIVE WELD SIZES AND MUST BE INCREASED TO ACCOUNT FOR ANY GAPS OR SKEWS BETWEEN MEMBERS AS REQUIRED BY ANSI/AWS D1.1.	
SC.4	BOLTED CONNECTIONS SHALL USE PRE-TENSIONED A490-N BOLTS IN ACCORDANCE WITH AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS".	
SC.5	ALL BOLTS SHALL BE 3/4" DIAMETER OR GREATER, UNLESS NOTED. USE DIRECT TENSION INDICATOR WASHERS FOR ALL BOLTED CONNECTIONS, UNLESS NOTED.	
SC.6	BOLTS THROUGH 4" WIDE BEAM FLANGES SHALL BE 5/8" DIAMETER.	
SC.7	BOLTS LOADED IN TENSION SHALL BE FULLY PRETENSIONED ACCORDING TO RCSC.	
SC.8	DO NOT REUSE PRETENSIONED BOLTS.	
PN. PAINT		
PN.1	BASIS OF DESIGN: PAINT MATERIALS SPECIFIED HEREIN ARE MANUFACTURED BY PPG INDUSTRIES, INC., AND ARE APPROVED FOR USE ON THIS PROJECT.	
PN.2	MATERIALS SPECIFIED ARE THOSE THAT HAVE BEEN EVALUATED FOR THE SPECIFIC SERVICE PRODUCTS OF PPG INDUSTRIES, INC. ARE LISTED TO ESTABLISH A STANDARD OF QUALITY EQUIVALENT MATERIAL OF OTHER MANUFACTURERS MAY BE SUBSTITUTED ON WRITTEN APPROVAL OF THE ENGINEER.	
PN.3	BIDDERS DESIRING TO USE COATINGS OTHER THAN THOSE SPECIFIED SHALL SUBMIT THEIR PROPOSAL IN WRITING TO THE ENGINEER. SUBSTITUTIONS THAT DECREASE THE FILM THICKNESS, THE NUMBER OF COATS APPLIED, CHANGE THE GENERIC TYPE OF COATING, OR FAIL TO MEET THE PERFORMANCE CRITERIA OF THE SPECIFIED MATERIALS WILL NOT BE APPROVED. PRIME AND FINISH COATS OF ALL SURFACES MUST BE FURNISHED BY THE SAME COATING MANUFACTURER.	
	A. MATERIALS SUPPLIED BY OTHER MANUFACTURERS MAY BE CONSIDERED FOR SUBSTITUTION IF THE FOLLOWING PREVAILING CONDITIONS EXIST:	
	1. PERFORMANCE CRITERIA OF THE PROPOSED MATERIALS ARE EQUAL AND DETAILED ON THE TECHNICAL DATA SHEETS OF EACH SPECIFIED PRODUCT.	
	2. THE SUBMITTAL MUST COMPARE RELEVANT PERFORMANCE CRITERIA OF THE SPECIFIED MATERIAL WITH THAT OF THE SUBMITTED ALTERNATE MATERIAL AND BE DOCUMENTED IN A SIDE-BY-SIDE COMPARISON MANNER FOR THE ENGINEER TO REVIEW.	
	3. SUBSTITUTE MATERIALS MUST BE FOR COMPLETE SYSTEMS AND NOT INDIVIDUAL PRODUCTS COMBINED WITH THE SPECIFIED MATERIALS AND THE PERFORMANCE CRITERIA FOR ALL PRODUCTS WITHIN A SYSTEM MUST MEET OR EXCEED THE SPECIFIED MATERIALS.	
PN.4	FINISH COAT COLOR OF NEW STRUCTURAL STEEL TO BE PSX700SG SW6991 BLACK MAGNETIC	
PN.5	SURFACE PREPARATION:	
	ALL STEEL SURFACES TO BE PREPARED IN ACCORDANCE WITH PPG 1490 CLEANING STEEL AND REMOVAL OF RUST STANDARD FROM PPG INDUSTRIES, INC.	

PN.6 STRIPE COATING:

APPLY BY BRUSH A STRIPE COAT TO ALL SHARP EDGES, BOLT HOLES AND WELDS, AS WELL AS LOCALIZED AREAS WHERE ACCESS WITH SPRAY EQUIPMENT IS DIFFICULT, SUCH AS ON THE BACK OF ANGLES OR ON THE REVERSE OF STIFFENERS.

NOTE: ANY HOLD BACK AREAS NECESSARY FOR WELDING (VERSUS BOLT CONNECTIONS) SHALL RECEIVE THE SPECIFIED SURFACE PREPARATION AND THEN BE IMMEDIATELY COATED WITH AWS CERTIFIED DEOXALUMINATE WELDABLE PRIMER. DEOXALUMINATE MUST BE APPLIED 1.0 - 1.5 MILS DFT.

PN.7 SHOP PRIMER:

AMERCOAT 240/SIGMACOAT 240 - PER MANUFACTURER'S SPECIFICATIONS
(CONTRACTOR SHALL SUBMIT PRODUCT DATA TO OWNER FOR REVIEW AND APPROVAL)

PN.8 FINISH COAT (CAN BE APPLIED IN THE SHOP OR IN THE FIELD, CANNOT BE SPRAYED IF APPLIED IN THE FIELD):

PSX700 SG - PER MANUFACTURER'S SPECIFICATIONS
(CONTRACTOR SHALL SUBMIT PRODUCT DATA TO OWNER FOR REVIEW AND APPROVAL)

PN.9 HANDLING OF SHOP COATED STRUCTURAL STEEL

A. STACKING AND SHIPPING OF SHOP PRIMED STEEL

1. SHOP PRIMED STEEL: KEEP PRIMED STEEL OFF THE GROUND BY PLACING ON WOODEN SUPPORTS AND KEEP MEMBERS FROM TOUCHING EACH OTHER BY USING WOODEN SEPARATORS FOR STACKING.
2. TAKE MEASURES TO AVOID DAMAGING SHOP APPLIED COATING SYSTEM WHILE STACKING, LOADING OR UNLOADING AND USE WOODEN PROTECTORS TO PREVENT DAMAGE FROM CHAIN OR CABLES CINCHES.

PN.10 FIELD TOUCH-UP:

A. AFTER ERECTION, ALL DAMAGED AREAS IN SHOP APPLIED COATING SYSTEM: RUST, EXPOSED SURFACES OF BOLTS, NUTS AND WASHERS, ALL FIELD WELDS, ABRADED AREAS AND UNPAINTED AREAS SHALL BE CLEANED TO THE SAME STANDARDS AS THE SHOP COAT. ALL FIELD COATED SURFACES SHALL RECEIVE THE SAME SHOP APPLIED GULF COAST PAINT COATING SYSTEM PRODUCT APPLIED TO THE SAME TOTAL DRY FILM THICKNESSES.

B. BOLT AND FASTENERS ASSEMBLIES

1. ALL BOLTS MUST BE DEGRADED BEFORE PAINTING SSPC SURFACE PREPARATION. AFTER DEGREASING ALL BOLTS MUST BE CLEANED IN ACCORDANCE WITH SSPC-SP2 WIRE BRUSH OR SSPC-SP3 POWER TOOL CLEAN STANDARDS.
2. ALL INSTALLED BOLTS AND BOLT ASSEMBLIES (WASHERS, LOCK WASHERS, NUTS, ETC.) MUST BE PRIMED BY BRUSH WITH 2.0-3.0 MILS DFT OF ZINC CLAD 4100 AND THEN TOP COATED WITH SHERLOXANE 800.

FS. FIELD SURVEY OF ERECTED STRUCTURAL STEEL FRAMING

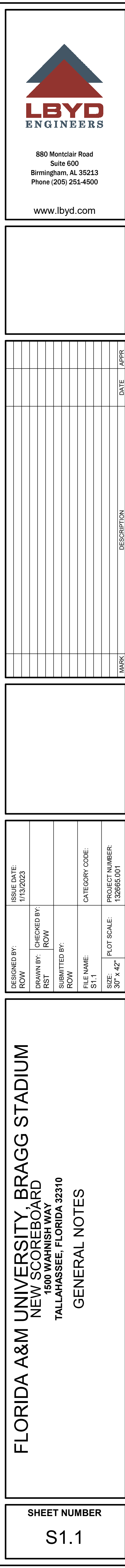
FS.1 AFTER STRUCTURAL STEEL ERECTION IS COMPLETE, CONTRACTOR TO SURVEY HORIZONTAL LOCATION OF ALL STRUCTURAL STEEL GIRTS THAT WILL BE SUPPORTING THE SCOREBOARD. SURVEY INFORMATION TO BE PROVIDED TO ARCHITECT, STRUCTURAL ENGINEER AND CONSTRUCTION MANAGER. THIS SURVEY INFORMATION WILL BE PROVIDED TO THE SCOREBOARD MANUFACTURER PRIOR TO BOARD INSTALLATION.

FP. FALL PROTECTION

FP.1 THE CONTRACTOR SHALL PROVIDE ENGINEERED FALL PROTECTION SYSTEMS FOR ACCESS TO ALL CATWALKS AND FOR MAINTENANCE SERVICING OF THE VIDEOBOARD UNITS.

FP.2 THE CONTRACTOR SHALL SUBMIT DRAWINGS OF THE FALL PROTECTION SYSTEMS FOR REVIEW BY THE STRUCTURAL ENGINEER.

FP.3 CALCULATIONS FOR THE FALL PROTECTION SYSTEMS SHALL BEAR THE SEAL OF A REGISTERED PROFESSIONAL ENGINEER IN THE STATE OF FLORIDA.



Special Inspection General Notes

SPECIAL INSPECTIONS

SI.1 ALL SPECIAL INSPECTIONS SHALL BE PERFORMED IN CONFORMANCE WITH THE APPLICABLE INTERNATIONAL BUILDING CODE AND ITS REFERENCED SPECIFICATIONS.

SI.2 THE SPECIAL INSPECTOR SHALL BE EMPLOYED BY THE OWNER OR THE OWNER'S AGENT AND NOT BY THE CONTRACTOR OR SUBCONTRACTOR WHOSE WORK IS TO BE INSPECTED OR TESTED. ANY CONFLICT OF INTEREST MUST BE DISCLOSED TO THE BUILDING OFFICIAL PRIOR TO COMMENCING WORK.

ST.3 THE SPECIAL INSPECTOR SHALL BE QUALIFIED PER THE INTERNATIONAL BUILDING CODE AND SHALL BE EDUCATED IN THE TASKS REQUIRED TO CONDUCT, SUPERVISE, AND EVALUATE THE INSPECTIONS. THE SPECIAL INSPECTOR MUST ALSO BE OBJECTIVE, COMPETENT, AND HAVE ACCESS TO THE APPROPRIATE TESTING EQUIPMENT WHICH SHALL BE MAINTAINED AND PERIODICALLY CALIBRATED. THE QUALIFICATIONS OF THE SPECIAL INSPECTOR MAY BE SUBJECT TO THE APPROVAL OF THE BUILDING OFFICIAL.

SI.4 SPECIAL INSPECTION AGENTS:

1. APPROVED TESTING AGENCY
2. GEOTECHNICAL ENGINEER OF RECORD:
FIRM
STREET ADDRESS
CITY, STATE, AND ZIP

EOR: ENGINEER OF RECORD:
LBYD INC.
880 MONTCLAIR ROAD, SUITE 600
BIRMINGHAM, AL 35213

SI.5 THE SPECIAL INSPECTIONS SHALL BE PERFORMED IN ADDITION TO ANY OBSERVATIONS PERFORMED BY THE ENGINEER OF RECORD AND ANY INSPECTIONS PERFORMED BY THE BUILDING OFFICIAL.

SI.6 THE SPECIAL INSPECTOR SHALL MAINTAIN RECORDS AND PROVIDE THE REQUIRED DOCUMENTATION AS PRESCRIBED IN THE INTERNATIONAL BUILDING CODE, INCLUDING THE SUBMITTAL OF REPORTS TO THE BUILDING OFFICIAL AND THE DESIGNER OF RECORD.

SI.7 THE CONTRACTOR SHALL COORDINATE THE CONSTRUCTION SCHEDULE WITH THE SPECIAL INSPECTOR TO ALLOW FOR SPECIAL INSPECTIONS.

SI.8 CONSTRUCTION WHICH REQUIRES SPECIAL INSPECTIONS SHALL BE MAINTAINED IN SUCH A STATE AS TO ALLOW ACCESS FOR THE SPECIAL INSPECTOR UNTIL THE REQUIRED INSPECTIONS OR TESTS HAVE BEEN COMPLETED.

SI.9 ANY DEVIATIONS FOUND DURING THE SPECIAL INSPECTION PROCESS SHALL IMMEDIATELY BE BROUGHT TO THE ATTENTION OF THE DESIGNER OF RECORD. ALL DEVIATIONS MUST BE ADDRESSED PRIOR TO COMPLETION OF THE WORK.

SI.10 INSPECTION FREQUENCY:

- A. CONTINUOUS - SPECIAL INSPECTION BY THE SPECIAL INSPECTOR WHO IS PRESENT WHEN AND WHERE THE WORK TO BE INSPECTED IS BEING PERFORMED.
- B. PERIODIC - SPECIAL INSPECTION BY THE SPECIAL INSPECTOR WHO IS INTERMITTENTLY PRESENT WHERE THE WORK TO BE INSPECTED HAS BEEN OR IS BEING PERFORMED.
- C. OBSERVE - OBSERVE ITEMS ON A RANDOM BASIS. OPERATIONS NEED NOT BE DELAYED PENDING THESE INSPECTIONS.
- D. PERFORM - PERFORM TASKS FOR EACH JOINT, MEMBER, AND CONNECTION.

ST.11 SPECIAL INSPECTIONS FOR STRUCTURAL, LOAD-BEARING, OR LATERAL LOAD BEARING FABRICATED ITEMS SHALL BE PERFORMED FOR THE FABRICATED ITEMS AT THE FABRICATOR'S SHOP. SPECIAL INSPECTIONS FOR FABRICATED ITEMS MAY BE WAIVED WHEN THE FABRICATOR IS REGISTERED AND HAS APPROVAL TO PERFORM THE WORK WITHOUT SPECIAL INSPECTIONS. IF THE INSPECTIONS ARE WAIVED, THE FABRICATOR MUST SUBMIT A CERTIFICATE OF COMPLIANCE TO THE BUILDING OFFICIAL SHOWING COMPLIANCE WITH THE APPROVED STRUCTURAL DRAWINGS.

SI.12 LATERAL SYSTEM NOTES

- A. THE MAIN WIND FORCE RESISTING SYSTEM SUBJECT TO SPECIAL INSPECTIONS IS COMPRISED OF:
1. ROOF DIAPHRAGM: STEEL HORIZONTAL BRACING
 2. FLOOR DIAPHRAGM: STEEL HORIZONTAL BRACING
 3. COLLECTOR ELEMENTS/DIAG STRUTS: N/A
 4. LATERAL FORCE RESISTING SYSTEM: CANTILEVERED STEEL COLUMNS

Soils

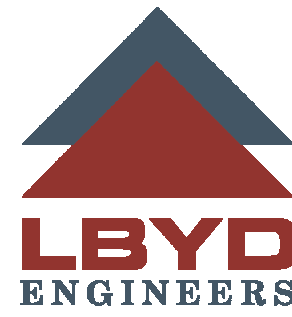
Soils				
NO.	INSPECTION TASK	FREQUENCY	REFERENCE FOR CRITERIA	AGENT
1.00	VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	PERIODIC		GEOR
2.00	VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	PERIODIC		GEOR
3.00	PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.	PERIODIC		GEOR
4.00	VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT, AND COMPACTION OF COMPACTED FILL.	CONTINUOUS		GEOR
5.00	PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	PERIODIC		GEOR

Concrete

Concrete				
NO.	INSPECTION TASK	FREQUENCY	REFERENCE STANDARD	AGENT
1.00	INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT.	PERIODIC	ACI 318 CH 20, 25.2, 25.3, 26.5.1-26.5.3; IBC 1908.4	ATA
2.00	REINFORCING BAR WELDING:			ATA
2.01	VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A 706.	PERIODIC	AWS D1.4	ATA
2.02	INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16".	PERIODIC	ACI 318: 26.5.4	ATA
2.03	INSPECT ALL OTHER WELDS.	CONTINUOUS		ATA
3.00	INSPECT ANCHORS CAST IN CONCRETE.	PERIODIC	ACI 318: 17.8.2	ATA
4.00	VERIFY USE OF REQUIRED DESIGN MIX.	PERIODIC	ACI 318: CH 19, 26.4.3, 26.4.4; IBC 1904.1, 1904.2, 1908.1, 1908.3	ATA
5.00	PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS; PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE. DETERMINE UNIT WEIGHT OF LIGHTWEIGHT CONCRETE.	CONTINUOUS	ASTM C 172; ASTM C 31; ACI 318:26.4.5, 26.12; IBC 1908.10	ATA
6.00	INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	CONTINUOUS	ACI 318: 26.4.5; IBC 1908.6, 1908.7, 1908.8	ATA
7.00	VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	PERIODIC	ACI 318: 26.4.7-26.4.9; IBC 1908.9	ATA

Structural Steel

NO.	INSPECTION TASK	FREQUENCY	REFERENCE FOR CRITERIA	AGENT
1.00	INSPECTOR SHALL BE ON THE PREMISES FOR INSPECTION DURING THE PLACEMENT OF ANCHOR RODS AND OTHER EMBEDMENTS SUPPORTING STRUCTURAL STEEL		AISC 360 SEC. N5.7	ATA
1.01	DIAMETER, GRADE, TYPE, LENGTH, AND EMBEDMENT DEPTH OF ANCHOR RODS AND OTHER EMBEDDED ITEMS	PERFORM		ATA
1.02	INSPECT THE FABRICATED STEEL OR ERECTED STEEL FRAME, AS APPROPRIATE, TO VERIFY COMPLIANCE WITH THE DETAILS SHOWN, SUCH AS BRACES, STIFFENERS, MEMBER LOCATIONS AND PROPER APPLICATION OF JOINT DETAILS AT EACH CONNECTION.	PERFORM		ATA
2.00	INSPECTION TASKS PRIOR TO WELDING:		AISC 360 SEC. N5.4	
2.01	WELDING PROCEDURE SPECIFICATIONS (WPS) AVAILABLE	PERFORM		ATA
2.02	MANUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE	PERFORM		ATA
2.03	MATERIAL IDENTIFICATION (TYPE/GRADE)	OBSERVE		ATA
2.04	WELDER IDENTIFICATION SYSTEM (a)	OBSERVE		ATA
2.05	FIT-UP OF GROOVE WELDS (INCLUDING JOINT GEOMETRY), JOINT PREPARATION, DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL), CLEANLINESS (CONDITION OF STEEL SURFACES), TACKING (TACK WELD QUALITY AND LOCATION), AND BACKING TYPE AND FIT (IF APPLICABLE)	OBSERVE		ATA
2.06	CONFIGURATION AND FINISH OF ACCESS HOLES	OBSERVE		ATA
2.07	CHECK WELDING EQUIPMENT	OBSERVE		ATA
3.00	INSPECTION TASKS DURING WELDING:		AISC 360 SEC. N5.4	
3.01	USE OF QUALIFIED WELDERS	OBSERVE		ATA
3.02	CONTROL AND HANDLING OF WELDING CONSUMABLE PACKAGING AND EXPOSURE CONTROL	OBSERVE		ATA
3.03	NO WELDING OVER CRACKED TACK WELDS	OBSERVE		ATA
3.04	ENVIRONMENTAL CONDITIONS INCLUDING WIND SPEED WITHIN LIMITS, PRECIPITATION, AND TEMPERATURE	OBSERVE		ATA
3.05	WPS FOLLOWED INCLUDING SETTINGS ON WELDING EQUIPMENT, TRAVEL SPEED, SELECTED WELDING MATERIALS, SHIELDING GAS TYPE/FLOW RATE, PREHEAT APPLIED, INTERPASS TEMPERATURE MAINTAINED (MIN/MAX), AND PROPER POSITION (F, V, H, OH)	OBSERVE		ATA
3.06	WELDING TECHNIQUES INCLUDING: INTERPASS AND FINAL CLEANING, EACH PASS WITHIN PROFILE LIMITATIONS, EACH PASS MEETS QUALITY REQUIREMENTS	OBSERVE		ATA
4.00	INSPECTION TASKS AFTER WELDING:		AISC 360 SEC. N5.4	
4.01	WELDS CLEANED	OBSERVE		ATA
4.02	SIZE, LENGTH, AND LOCATION OF WELDS	PERFORM		ATA
4.03	WELDS MEET VISUAL ACCEPTANCE CRITERIA FOR: CRACK PROHIBITION, WELD/BASE-METAL FUSION, CRATER CROSS SECTION, WELD PROFILES, WELD SIZE, UNDERCUT, AND POROSITY	PERFORM		ATA
4.04	ARC STRIKES	PERFORM		ATA
4.05	K-AREA (b)	PERFORM		ATA
4.06	BACKING REMOVED AND WELD TABS REMOVED (IF REQUIRED)	PERFORM		ATA
4.07	REPAIR ACTIVITIES	PERFORM		ATA
5.00	INSPECTION TASKS PRIOR TO BOLTING:		AISC 360 SEC. N5.6	
5.01	MANUFACTURER'S CERTIFICATIONS AVAILABLE FOR FASTENER MATERIALS	PERFORM		ATA
5.02	FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS	OBSERVE		ATA
5.03	PROPER FASTENERS SELECTED FOR THE JOINT DETAIL (GRADE, TYPE, BOLT LENGTH IF THREADS ARE TO BE EXCLUDED FROM SHEAR PLANE)	OBSERVE		ATA
5.04	PROPER BOLTING PROCEDURE SELECTED FOR JOINT DETAIL	OBSERVE		ATA
5.05	CONNECTING ELEMENTS, INCLUDING THE APPROPRIATE FAYING SURFACE CONDITION AND HOLE PREPARATION, IF SPECIFIED, MEET APPLICABLE REQUIREMENTS	OBSERVE		ATA
5.06	PRE-INSTALLATION VERIFICATION TESTING BY INSTALLATION PERSONNEL OBSERVED AND DOCUMENTED FOR FASTENER ASSEMBLIES AND METHODS USED	OBSERVE		ATA
5.07	PROPER STORAGE PROVIDED FOR BOLTS, NUTS, WASHERS AND OTHER FASTENER COMPONENTS	OBSERVE		ATA
6.00	INSPECTION TASKS DURING BOLTING:		AISC 360 SEC. N5.6	
6.01	FASTENER ASSEMBLIES, OF SUITABLE CONDITION, PLACED IN ALL HOLES AND WASHERS (IF REQUIRED) ARE POSITIONED AS REQUIRED	OBSERVE		ATA
6.02	JOINT BROUGHT TO THE SNUG-TIGHT CONDITION PRIOR TO THE PRETENSIONING OPERATION	OBSERVE		ATA
6.03	FASTENER COMPONENT NOT TURNED BY THE WRENCH PREVENTED FROM ROTATING	OBSERVE		ATA
6.04	FASTENERS ARE PRETENSIONED IN ACCORDANCE WITH THE RCSC SPECIFICATION, PROGRESSING SYSTEMATICALLY FROM THE MOST RIGID POINT TOWARD THE FREE EDGES	OBSERVE		ATA
7.00	INSPECTION TASKS AFTER BOLTING:		AISC 360 SEC. N5.6	
7.01	DOCUMENT ACCEPTANCE OR REJECTION OF BOLTED CONNECTIONS	PERFORM		ATA
NOTES:	(a) THE FABRICATOR OR ERECTOR, AS APPLICABLE, SHALL MAINTAIN A SYSTEM BY WHICH A WELDER WHO HAS WELDED A JOINT OR MEMBER CAN BE IDENTIFIED. STAMPS, IF USED, SHALL BE THE LOW-STRESS TYPE.			
	(b) WHEN WELDING OF DOUBLER PLATES, CONTINUITY PLATES OR STIFFENERS HAS BEEN PERFORMED IN THE K-AREA, VISUALLY INSPECT THE WEB K-AREA FOR CRACKS WITHIN 3 IN. (75MM) OF THE WELD.			



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[illegible]

DESIGNED BY: ROW	ISSUE DATE: 1/13/2023	
DRAWN BY: RST	CHECKED BY: ROW	
SUBMITTED BY: ROW		
FILE NAME: S12	CATEGORY CODE:	
SIZE: 30" x 42"	PLOT SCALE: 3/4" = 1'-0"	PROJECT NUMBER: 132665-001

FLORIDA A&M UNIVERSITY, BRAGG STADIUM
NEW SCOREBOARD
1500 WAINWICK WAY
TALLAHASSEE, FLORIDA 32310
SPECIAL INSPECTIONS

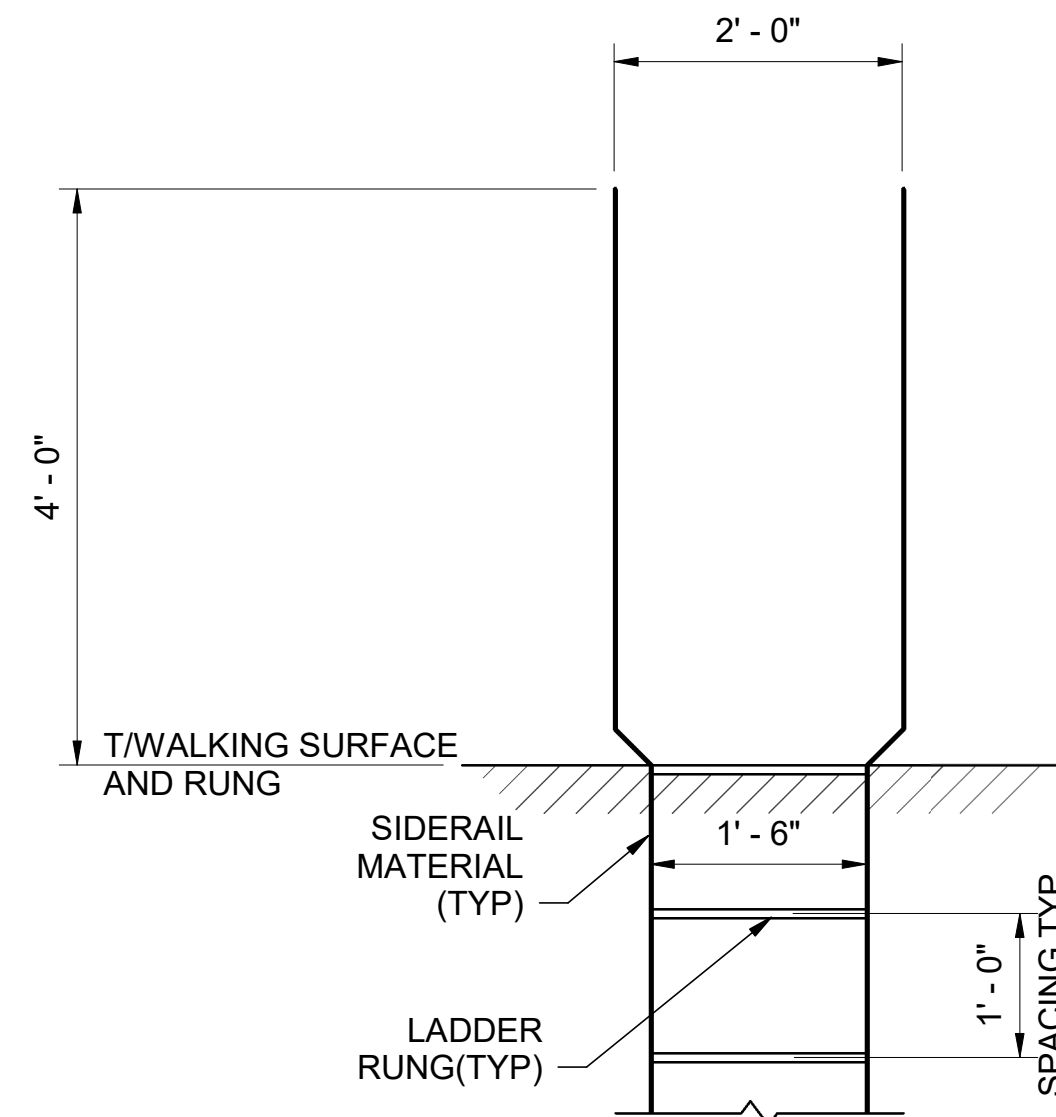
SHEET NUMBER

S1.2

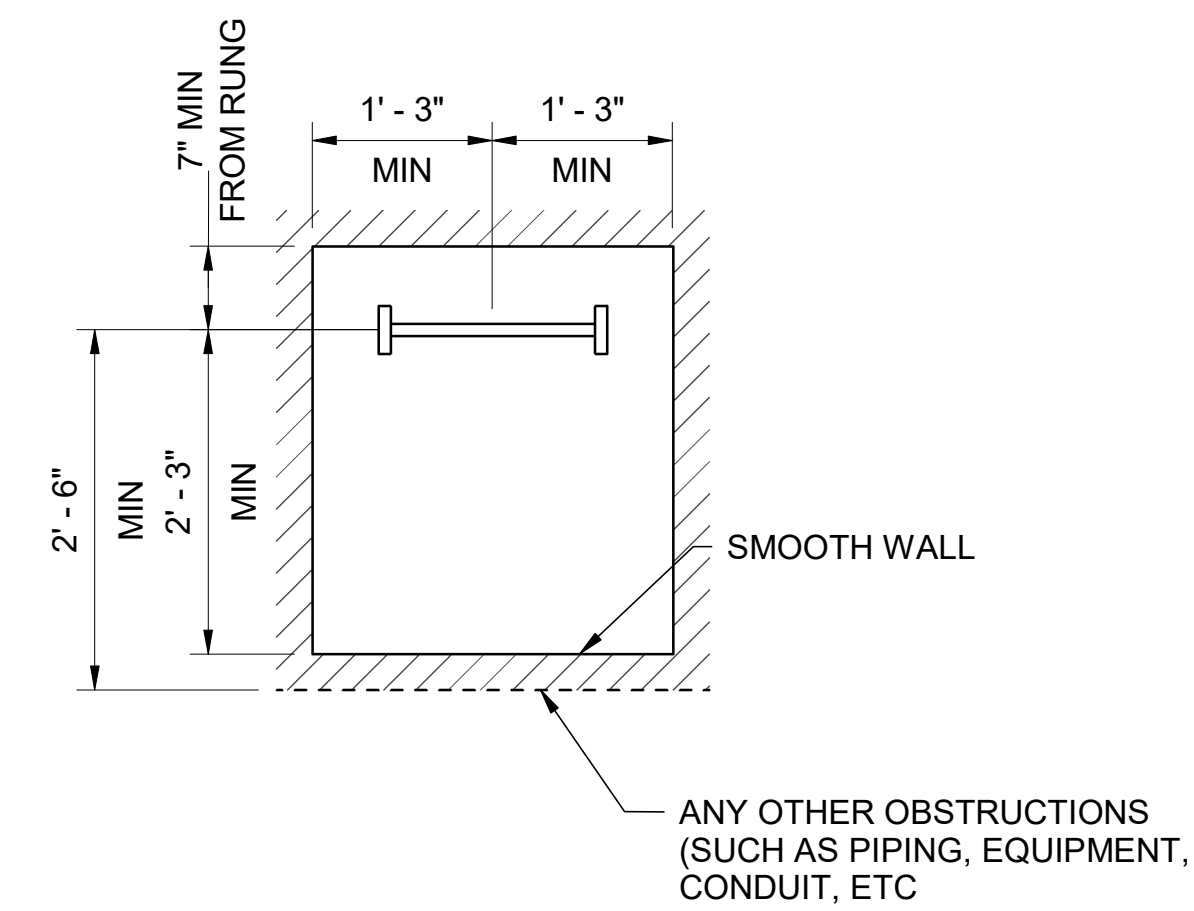
INITIAL PRICING SET - NOT FOR CONSTRUCTION

Tension Lap Splice Lengths												
BAR SIZE	f _C = 3000				f _C = 4000				f _C = 5000			
	TOP BARS		OTHER BARS		TOP BARS		OTHER BARS		TOP BARS		OTHER BARS	
	A	B	A	B	A	B	A	B	A	B	A	B
#3	22"	28"	17"	22"	19"	24"	15"	19"	17"	22"	13"	17"
#4	29"	37"	22"	29"	25"	32"	19"	25"	22"	29"	17"	22"
#5	36"	47"	28"	36"	31"	40"	24"	31"	28"	36"	22"	28"
#6	43"	56"	33"	43"	37"	48"	29"	37"	33"	43"	26"	33"
#7	63"	81"	48"	63"	54"	70"	42"	54"	49"	63"	37"	49"
#8	72"	93"	55"	72"	62"	80"	48"	62"	55"	72"	43"	55"
#9	81"	105"	62"	81"	70"	91"	54"	70"	63"	81"	48"	63"
#10	91"	118"	70"	91"	79"	102"	61"	79"	70"	91"	54"	70"
#11	101"	131"	78"	101"	87"	113"	67"	87"	78"	101"	60"	78"

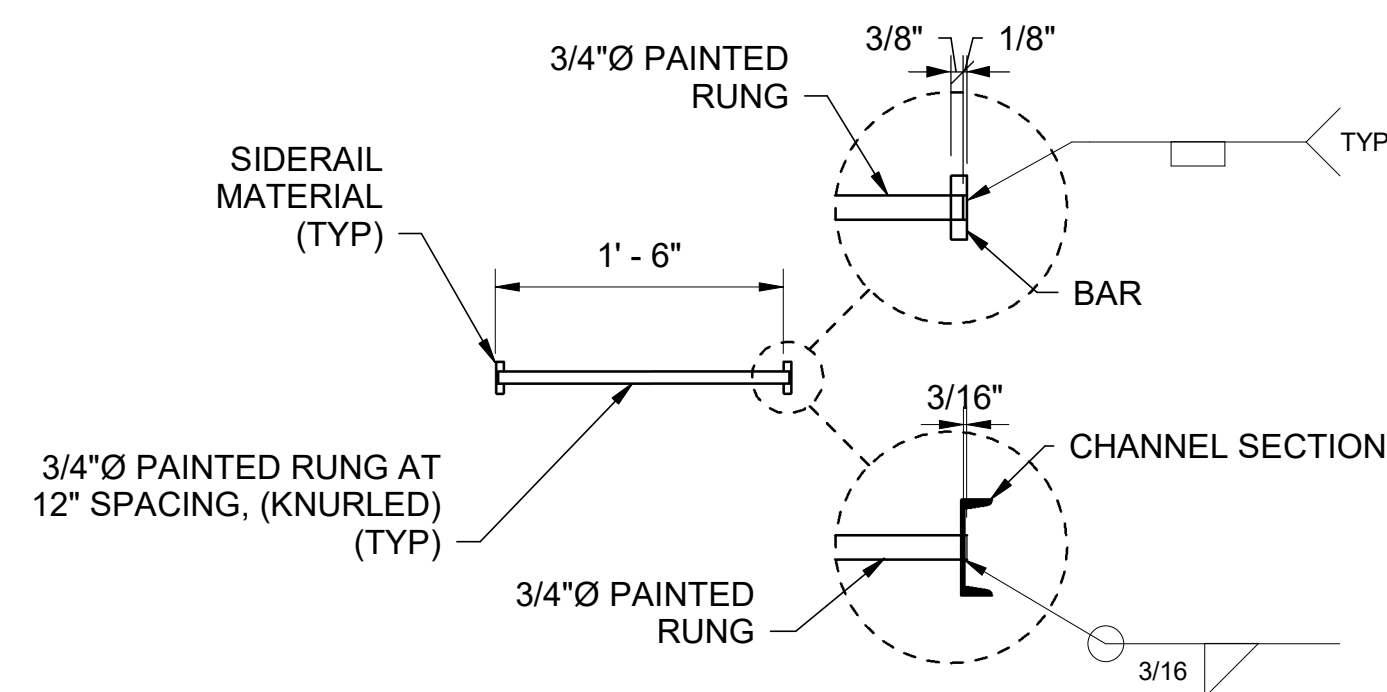
1. THIS TABLE CONTAINS DEVELOPMENT AND SPICE LENGTHS FOR NORMAL-WEIGHT CONCRETE SLABS ONLY.
2. ALL DEVELOPMENT/SPICE LENGTHS ARE IN INCHES (IN.).
3. LAP DEVELOPMENT LENGTH, PER CHAPTER 12, FCI 318.
4. SLABS SHALL APPLY WHEN ACI 318 MINIMUM COVER IS PROVIDED (THE GREATER OF 0.75" AND d_b) AND THE CENTER-TO-CENTER BAR SPACING IS $\geq 3d_b$.
5. WHEN LAP SPICING BARS OF DIFFERENT SIZES, THE LAP LENGTH IS DETERMINED BY THE SMALLER BAR, BUT SHALL NOT BE LESS THAN THE "CLASS A" SPICE LENGTH OF THE LARGER BAR.
6. THE DEVELOPMENT/SPICE LENGTH SHOWN IN THIS TABLE HAVE NOT BEEN FACTORED FOR EPOXY-COATED REBAR.
7. TOP BARS ARE HORIZONTAL REINFORCEMENT WITH MORE THAN 12" OF CONCRETE CAST BELOW THE REINFORCEMENT.



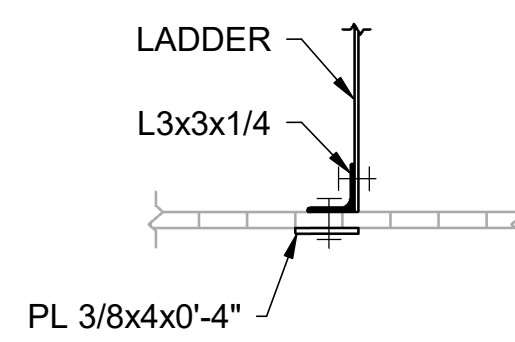
Typical Step-Thru Ladder



Min Ladder Clearances



Typical Ladder Detail



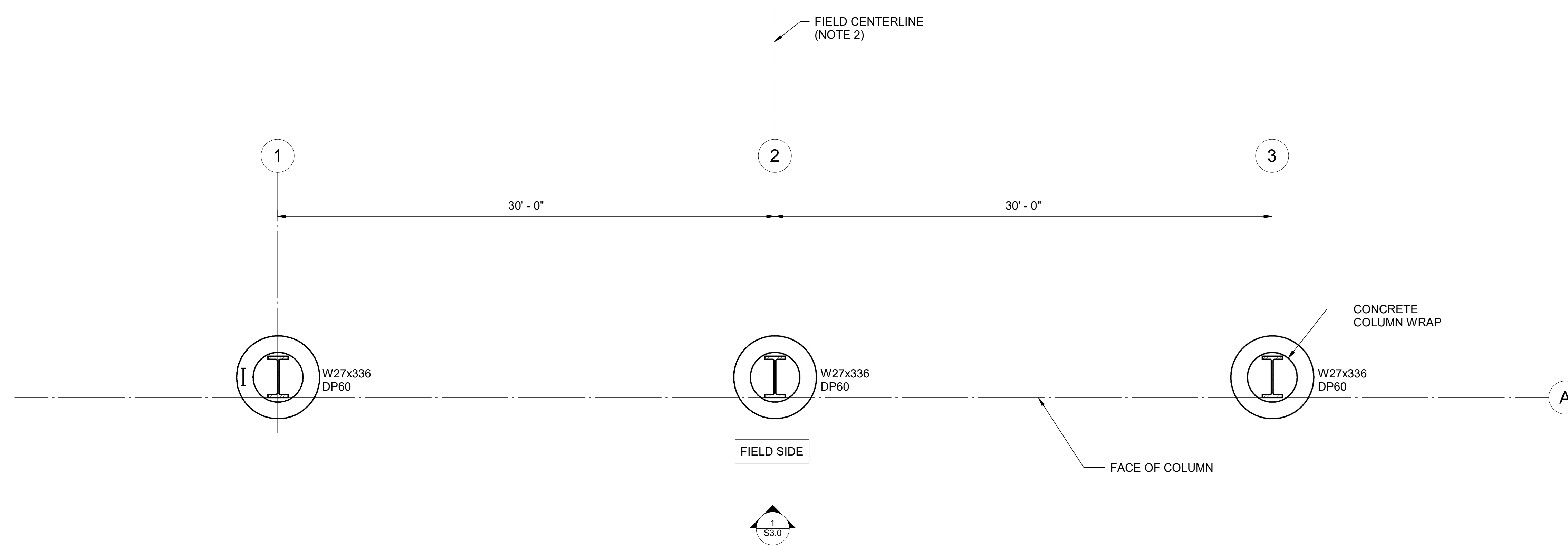
Typical Ladder Base Detail

DESIGNED BY: ROW		ISSUE DATE: 1/13/2023	
DRAWN BY: RST	CHECKED BY: ROW		
SUBMITTED BY: ROW			
FILE NAME: S1.3		CATEGORY CODE:	
SIZE: 30" x 42"	PLOT SCALE: As indicated	PROJECT NUMBER: 132655.001	

FLORIDA A&M UNIVERSITY, BRAGG STADIUM
NEW SCOREBOARD
1500 WAHNSHAW WAY
TALLAHASSEE, FLORIDA 32310
TYPICAL DETAILS

SHEET NUMBER

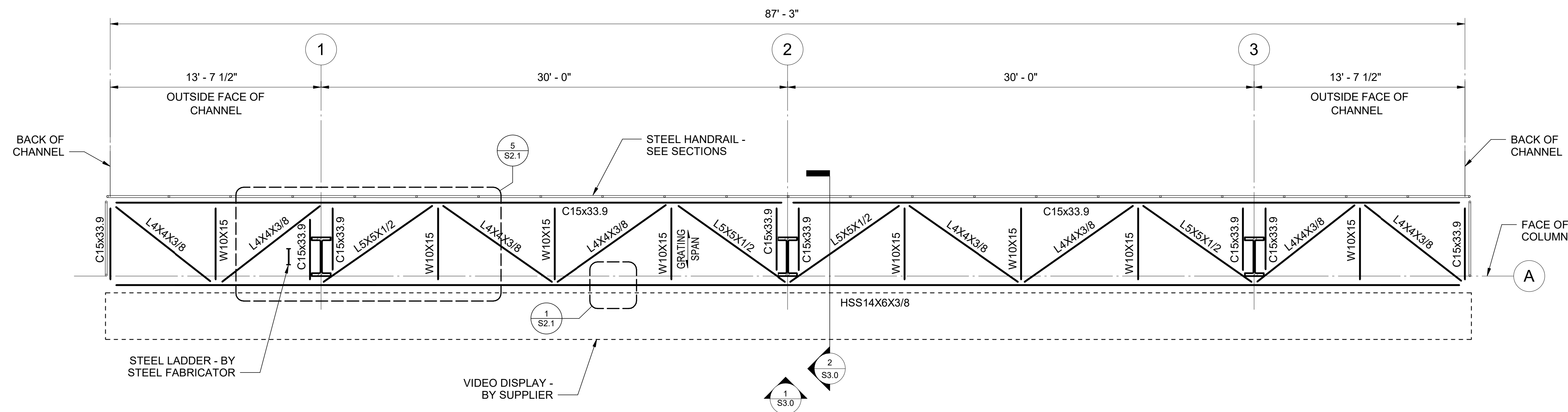
S1.3



Foundation Plan

$$1/4'' = 1'-0''$$

- NOTES:
1. FOUNDATION DESIGN SHOWN IS SCHEMATIC AND IS SUBJECT TO CHANGE. FOUNDATION DESIGN WILL BE FINALIZED UPON RECEIPT OF THE GEOTECHNICAL REPORT. CURRENT DESIGN CAN BE USED FOR PRICING AT CONTRACTOR'S RISK.
 2. FINAL LOCATION OF SCOREBOARD FOUNDATIONS SHALL BE COORDINATED WITH CIVIL DRAWINGS AND OWNER.
 3. 'DP' DENOTES DRILLED PIER.



Catwalk Framing Plan - First Level

$$1/4'' = 1'-0''$$

- NOTES:
1. STEEL BAR GRATING: 1 1/2" x 3/16" 19-W-4 HOT-DIPPED GALVANIZED
 2. SEE SECTIONS FOR TOP OF STEEL (TOS).
 3. TOS OF ALL ANGLE BRACING IS -3".

[illegible]

DESIGNED BY: ROW	ISSUE DATE: 1/13/2023	
	DRAWN BY: RST	CHECKED BY: ROW
SUBMITTED BY: ROW	CATEGORY CODE:	
FILE NAME: S2.0	SIZE: 30" x 42"	PLOT SCALE: 1/4" = 1'-0"
PROJECT NUMBER: 132665.001		

INITIAL PRICING SET - NOT FOR CONSTRUCTION

FLORIDA A&M UNIVERSITY, BRAGG STADIUM
NEW SCOREBOARD
1500 WAHNSH WAY

NEW COOKING AND
1500 WAHNISH WAY

FOUNDATION PLAN & CATWALK FRAMING PLAN - LEVEL 1

SHEET NUMBER

62.0



1/4" = 1'-0"

NOTES:

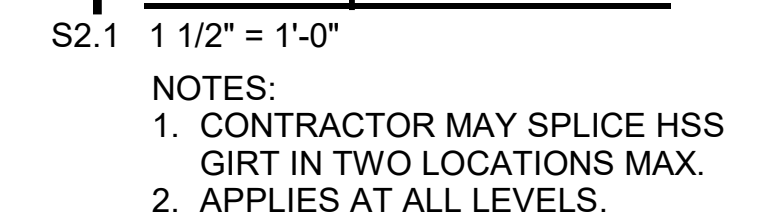
1. STEEL BAR GRATING: 1 1/2" x 3/16" 19-W-4 HOT-DIPPED GALVANIZED
2. SEE SECTIONS FOR TOP OF STEEL (TOS).
3. TOS OF ALL ANGLE BRACING IS -3".



1/4" = 1'-0"

NOTES:

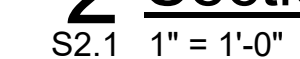
1. STEEL BAR GRATING: 1 1/2" x 3/16" 19-W-4 HOT-DIPPED GALVANIZED
2. SEE SECTIONS FOR TOP OF STEEL (TOS).
3. TOS OF ALL ANGLE BRACING IS -3".



1" = 1'-0"

NOTES:

1. BOLTS 1" OR 3/4"Ø A325, FULLY PRETENSIONED. STD HOLES ALL PLIES.
2. CONNECTION MATERIAL A36.
3. ALL WELDS 1/4" DOUBLE FILLET WELDS.



S3.0 $1/4'' = 1'-0''$



S3.0 $1/4" = 1'-0"$

[illegible]

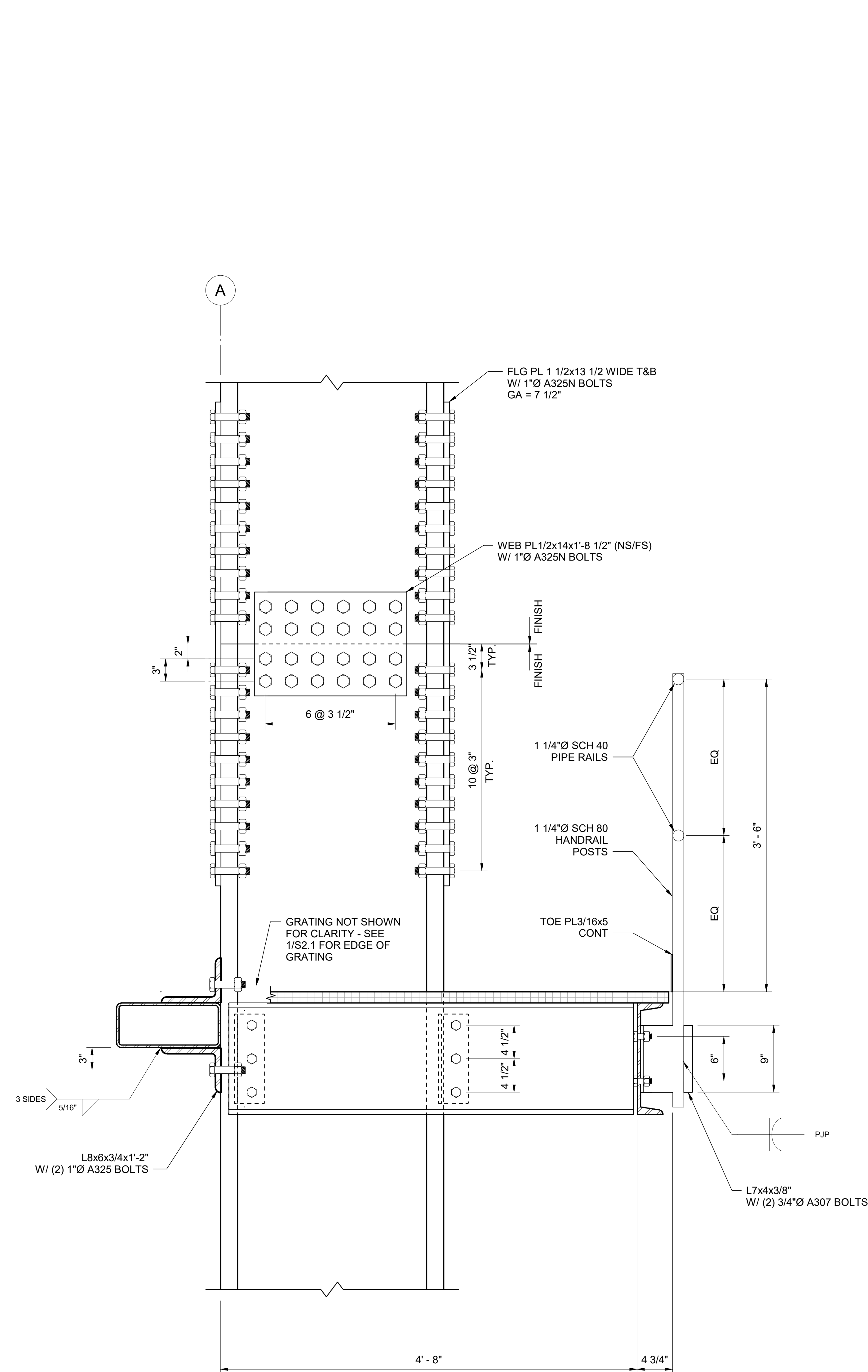
DESIGNED BY: Designer	ISSUE DATE: 01/13/23	
DRAWN BY: Author	CHECKED BY: Checker	
SUBMITTED BY: Approver		
FILE NAME: SS3.1	CATEGORY CODE:	
SIZE: 30" x 42"	PLOT SCALE: As Indicated	PROJECT NUMBER: 132685.001

FLORIDA A&M UNIVERSITY, BRAGG STADIUM
NEW SCOREBOARD
1500 WAINNISH WAY
TALLAHASSEE, FLORIDA 32310

SHEET NUMBER

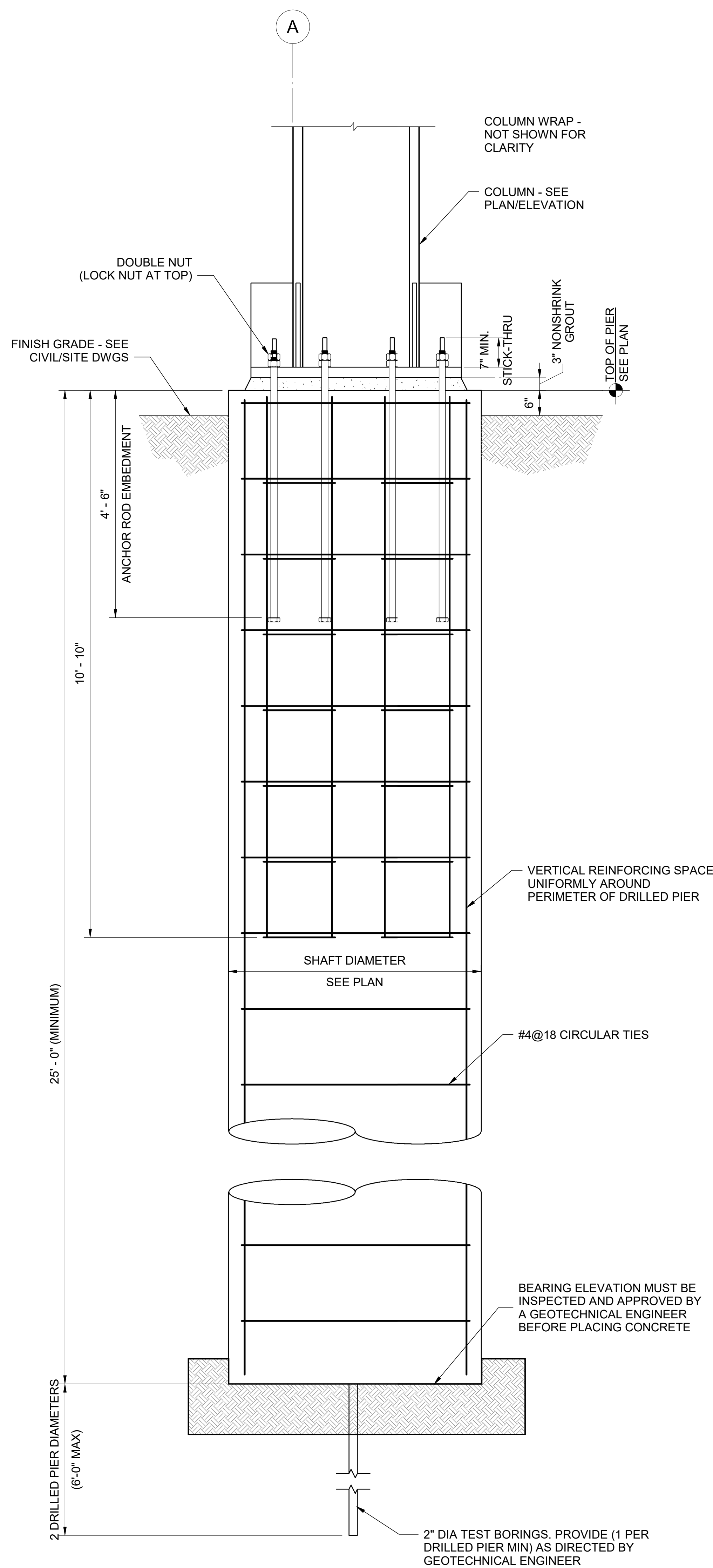
S3.1

INITIAL PRICING SET - NOT FOR CONSTRUCTION



1 Detail

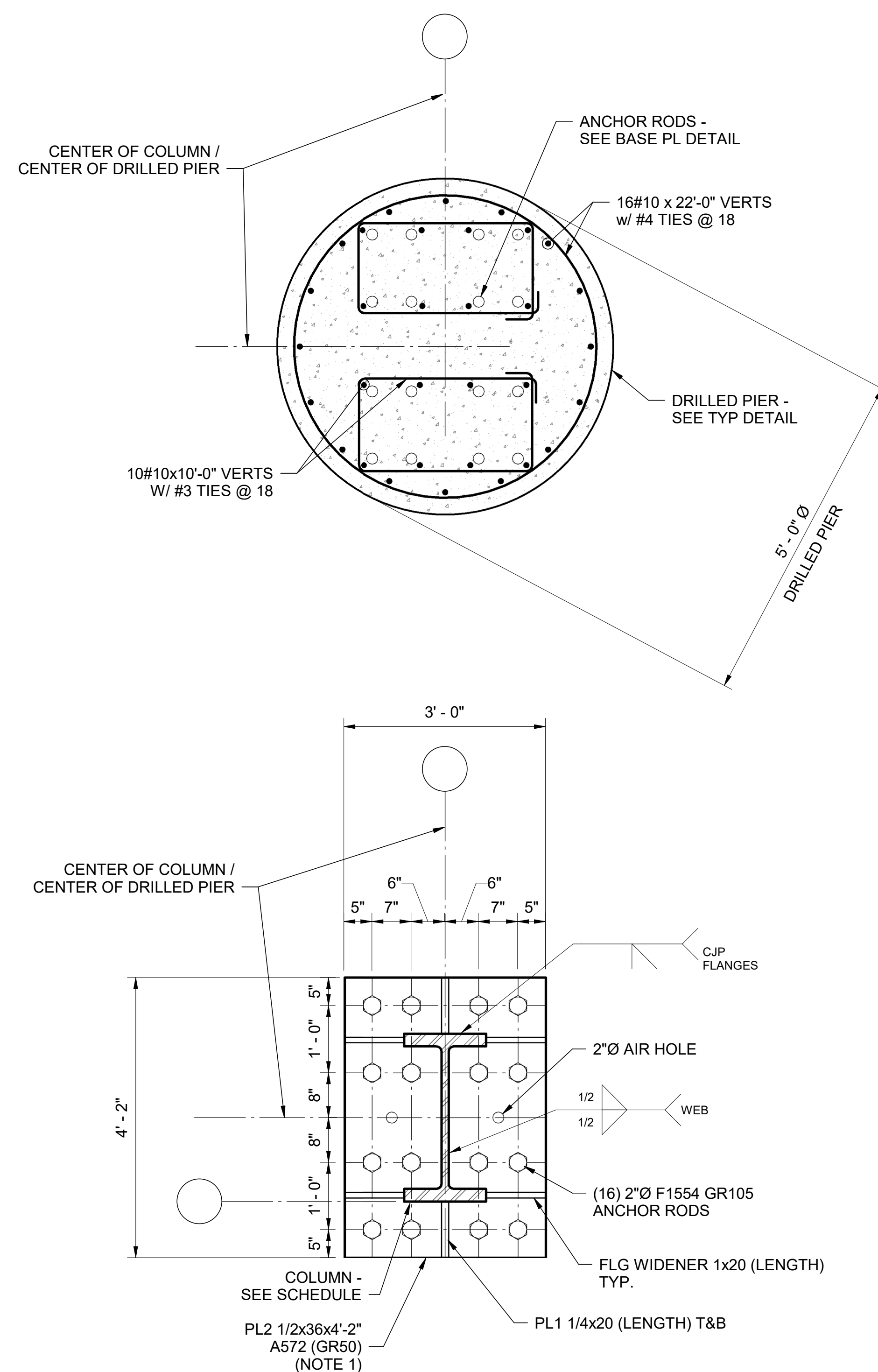
S3.1 1 1/2" = 1'-0"



2 Section

S3.1 $3/4" = 1'-0"$

NOTES:
1. FOUNDATION DESIGN SHOWN IS SCHEMATIC AND IS SUBJECT TO CHANGE. FOUNDATION DESIGN WILL BE FINALIZED UPON RECEIPT OF THE GEOTECHNICAL REPORT. CURRENT DESIGN CAN BE USED FOR PRICING AT CONTRACTOR'S RISK.



3 Column Base Plate & Drilled Pier Detail

S3.1 $3/4" = 1'-0"$

NOTES:

1. CONTRACTOR TO SURVEY AS-BUILT LOCATION OF ANCHOR RODS PRIOR TO STEEL DETAILING/FABRICATION.
2. A POST-INSTALLED SOLUTION IS NOT AN ACCEPTABLE OPTION.
3. FOUNDATION DESIGN SHOWN IS SCHEMATIC AND IS SUBJECT TO CHANGE. FOUNDATION DESIGN WILL BE FINALIZED UPON RECEIPT OF THE GEOTECHNICAL REPORT. CURRENT DESIGN CAN BE USED FOR PRICING AT CONTRACTOR'S RISK.